Improving Resilience of Moment Frames Using Steel Pipe Dampers

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Outline of the Presentation

1. Introduction
2. Steel pipe dampers
3. Passive energy dissipation systems
   - Damper configuration and earthquake time histories
   - Results of the simulation study
4. Improving physical resilience of moment resisting frames
5. Conclusion
Introduction

- **Earthquake resiliency** of moment resisting frames, either new or existing ones, are important for maintaining their functionality.
- **Steel pipe dampers** are capable to dissipate most of the earthquake energy in structures through inelastic deformation so that other components of the structures are protected.
- Steel pipe dampers, **when installed at strategic locations** in the moment frame structures, can be used to improve earthquake resiliency of moment resisting frames.
Steel Pipe Dampers

Steel pipes are chosen as the basic material for seismic dampers because:

1. Pipes have excellent inelastic deformation capability.
2. Pipes are cheap and require low workmanship.
Vertical Steel Pipe Dampers

Pipe 113.4 x 8.6 x 200 mm

Dimensions:
- 200 mm
- 16 mm
- 26 mm
- 20 mm
- 20 mm
- R=25 mm
- Plate 12 mm
- 9.5 mm
- 50 mm
- 20 mm
Curved and smoothed to avoid fracture

Welded connections area placed at low stress regions
Passive energy dissipation systems
(Christopoulos, C. and Filliatrault, A. (2006))
Damper configuration and earthquake loading
Damper configuration

This configuration is used to minimize the effect of axial forces.
Steel pipe dampers yield when they dissipate input earthquake energy in structures. Yielding steel pipe dampers loose most of their stiffneses. Steel pipe dampers are weak axially when they yield.
Earthquake time histories

North-South components of the following earthquakes will be used in numerical simulation.

1. El-Centro (California), 1940.
2. Fukushima-Hamadori (Japan), 2011
3. Padang (west Sumatra), 2009
4. Chi-Chi (Taiwan), 1999.

Peak Ground Acceleration of the three other earthquakes were scaled down to El-Centro 1940.
Result of the simulation study

**Shear Behavior**

\[ F = \text{shear and bearing forces}, \quad D = \text{shear and bearing deformations}. \]

**Capacity**

<table>
<thead>
<tr>
<th>Along Axis 1</th>
<th>Along Axis 2</th>
</tr>
</thead>
<tbody>
<tr>
<td>FY</td>
<td>197.55</td>
</tr>
<tr>
<td>FU</td>
<td>463.75</td>
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</table>

**Deformations**

<table>
<thead>
<tr>
<th>Along Axis 1</th>
<th>Along Axis 2</th>
</tr>
</thead>
<tbody>
<tr>
<td>DU</td>
<td>0.007751</td>
</tr>
<tr>
<td>DX</td>
<td>0.035</td>
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</tbody>
</table>
Four steps in the numerical study of the damper application:

1. Determine the basic frame structure.
2. Select the earthquake time histories used in the numerical simulation.
3. Determine the required number of dampers at each story.
4. Evaluate the effectiveness of seismic protection in the frame structure with dampers installed.
Inter-story drift due to four earthquakes: (a) Frame without dampers and (b) Frame with dampers
Dissipated inelastic energy in kN.m (along H1)

<table>
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<tr>
<th></th>
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<tr>
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<td>Perimeter Beams</td>
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<td>0</td>
</tr>
<tr>
<td>Interior Columns</td>
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</tr>
<tr>
<td>Interior Beams</td>
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<tr>
<td>Connection Panel Zones - along H1</td>
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<td>0</td>
<td>0</td>
<td>0</td>
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<tr>
<td>Connection Panel Zones - along H2</td>
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<tr>
<td>Vertical Steel Pipe Dampers</td>
<td>261.49</td>
<td>422.4</td>
<td>202.35</td>
<td>233.29</td>
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<td>Bracing HSS-H1-1st floor</td>
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<tr>
<td>Bracing HSS-H2-1st floor</td>
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<tr>
<td>Bracing HSS-H2-other floors</td>
<td>0</td>
<td>0</td>
<td>0</td>
<td>0</td>
</tr>
<tr>
<td>Bracing HSS-H1 other floors</td>
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<td>0</td>
<td>0</td>
<td>0</td>
</tr>
<tr>
<td><strong>Total for All Groups</strong></td>
<td><strong>261.49</strong></td>
<td><strong>422.4</strong></td>
<td><strong>202.35</strong></td>
<td><strong>233.29</strong></td>
</tr>
</tbody>
</table>
Time history of energy dissipation due to four earthquakes

**Inelastic Energy to Input Energy Ratio and Maximum Inelastic Energy**

- **Chi-Chi (duration 100 sec)**
  - Max. input energy: $E_{i,m} = 444.82 \text{ kN.m}$
  - Max. inelastic energy: $E_{h,m} = 233.32 \text{ kN.m}$

- **El-Centro ($E_h/E_{i,m}$)**

- **Padang ($E_h/E_{i,m}$)**

- **Fukushima ($E_h/E_{i,m}$)**

- **Padang (duration 40 sec)**
  - Max. input energy: $E_{i,m} = 450.39 \text{ kN.m}$
  - Max. inelastic energy: $E_{h,m} = 202.35 \text{ kN.m}$

- **Chi-Chi ($E_h/E_{i,m}$)**

- **El_Centro (duration 25 sec)**
  - Max. input energy: $E_{i,m} = 535.94 \text{ kN.m}$
  - Max. inelastic energy: $E_{h,m} = 262.61 \text{ kN.m}$

- **Fukushima (duration 100 sec)**
  - Max. input energy: $E_{i,m} = 888.56 \text{ kN.m}$
  - Max. inelastic energy: $E_{h,m} = 422.39 \text{ kN.m}$

**Time (second)**
Improving physical resilience of moment resisting frames

Hazards (corrosion induced damages and earthquakes) pose continuing and significant threats to structures by reducing the capability to withstand the effect of seismic event and to recover efficiently the original functionality of the structures.
Aging

Moderate EQ

Aging

Strong EQ

Recovery

Very strong EQ

Residual stiffness and strength

Collapse

Collapse

Local damage

CP

Structure Performance Level

10 yrs

20 yrs

30 yrs

40 yrs

50 yrs

Time
Fragility curves

Effects of resourcefulness and recovery phase on fragility curve: a) Functionality; b) Fragility curve
Conclusion

The use of steel pipe dampers improves the resiliency (reduces the overall earthquake response) of the moment frames, the installation of steel pipe dampers increases story shears, member forces around the dampers and floor accelerations.