

Improving Resilience of Moment Frames Using Steel Pipe Dampers

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Outline of the Presentation

1. Introduction
2. Steel pipe dampers
3. Passive energy dissipation systems
 - Damper configuration and earthquake time histories
 - Results of the simulation study
4. Improving physical resilience of moment resisting frames
5. Conclusion

Introduction

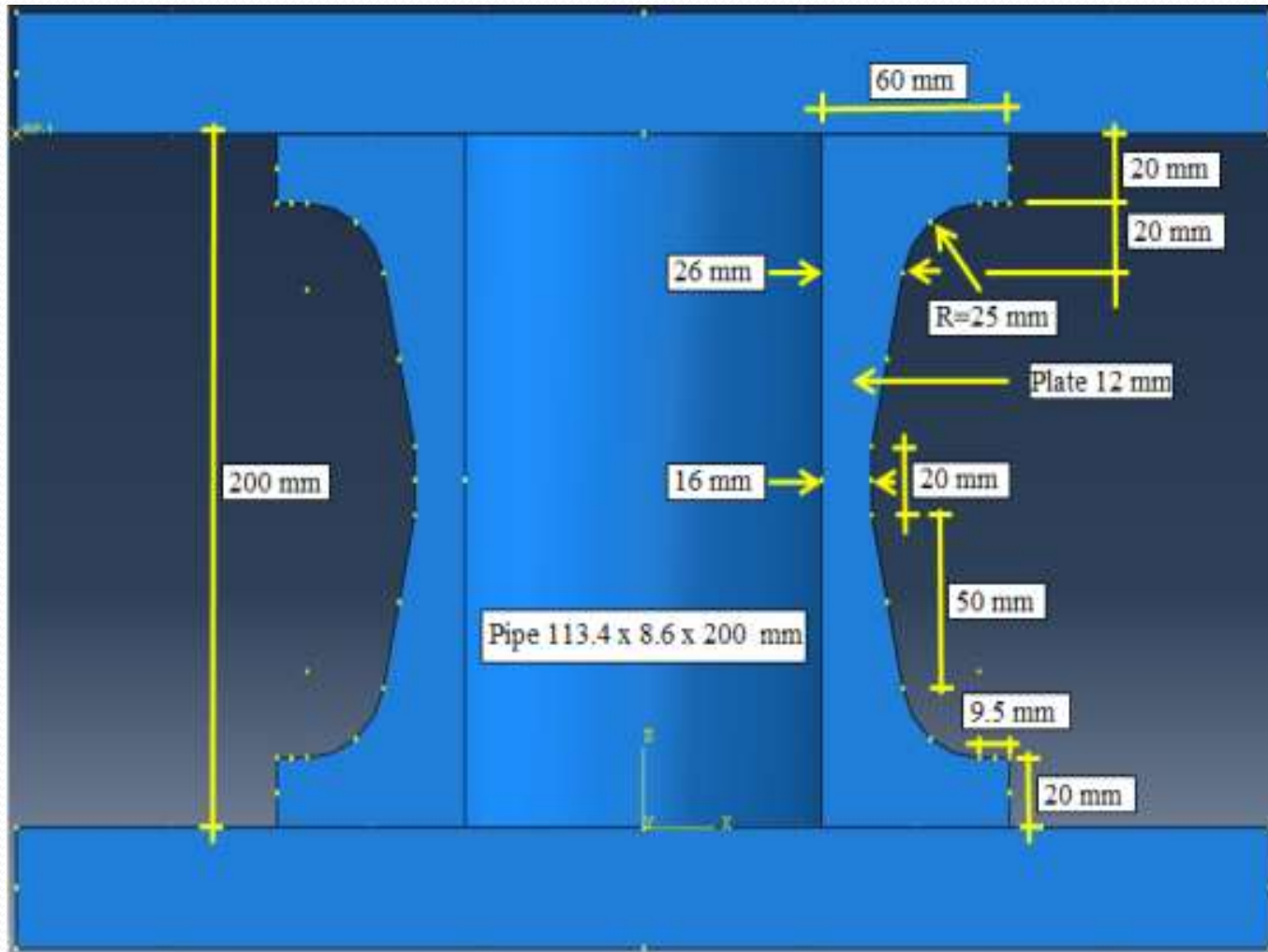
- **Earthquake resiliency** of moment resisting frames, either new or existing ones, are important for maintaining their functionality.
- **Steel pipe dampers** are capable to dissipate most of the earthquake energy in structures through inelastic deformation so that other components of the structures are protected.
- Steel pipe dampers, **when installed at strategic locations** in the moment frame structures, can be used to improve earthquake resiliency of moment resisting frames.

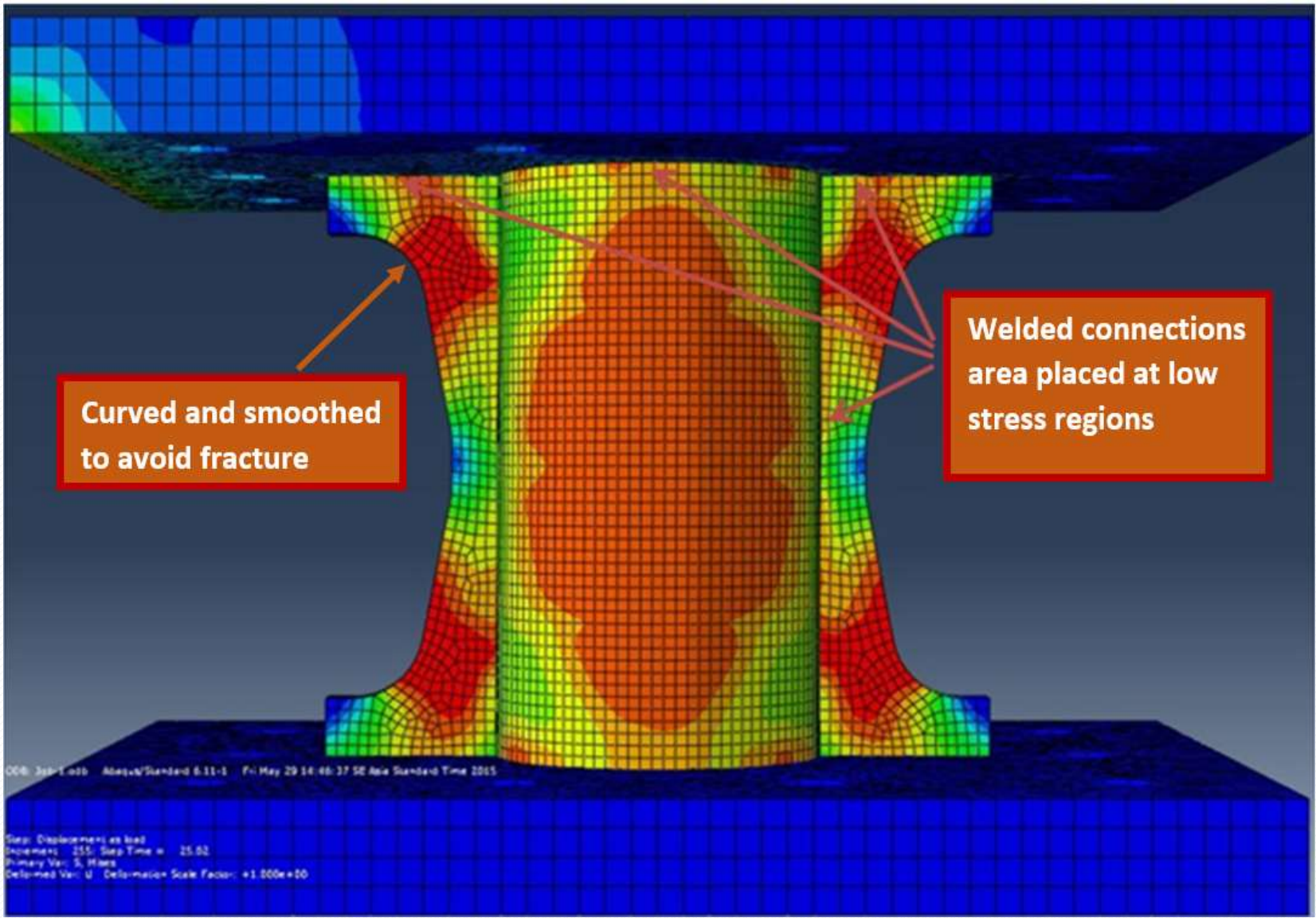
Steel Pipe Dampers

Steel pipes are chosen as the basic material for seismic dampers because:

1. Pipes have **excellent** inelastic deformation capability.
2. Pipes are **cheap** and require **low workmanship**.

Vertical Steel Pipe Dampers



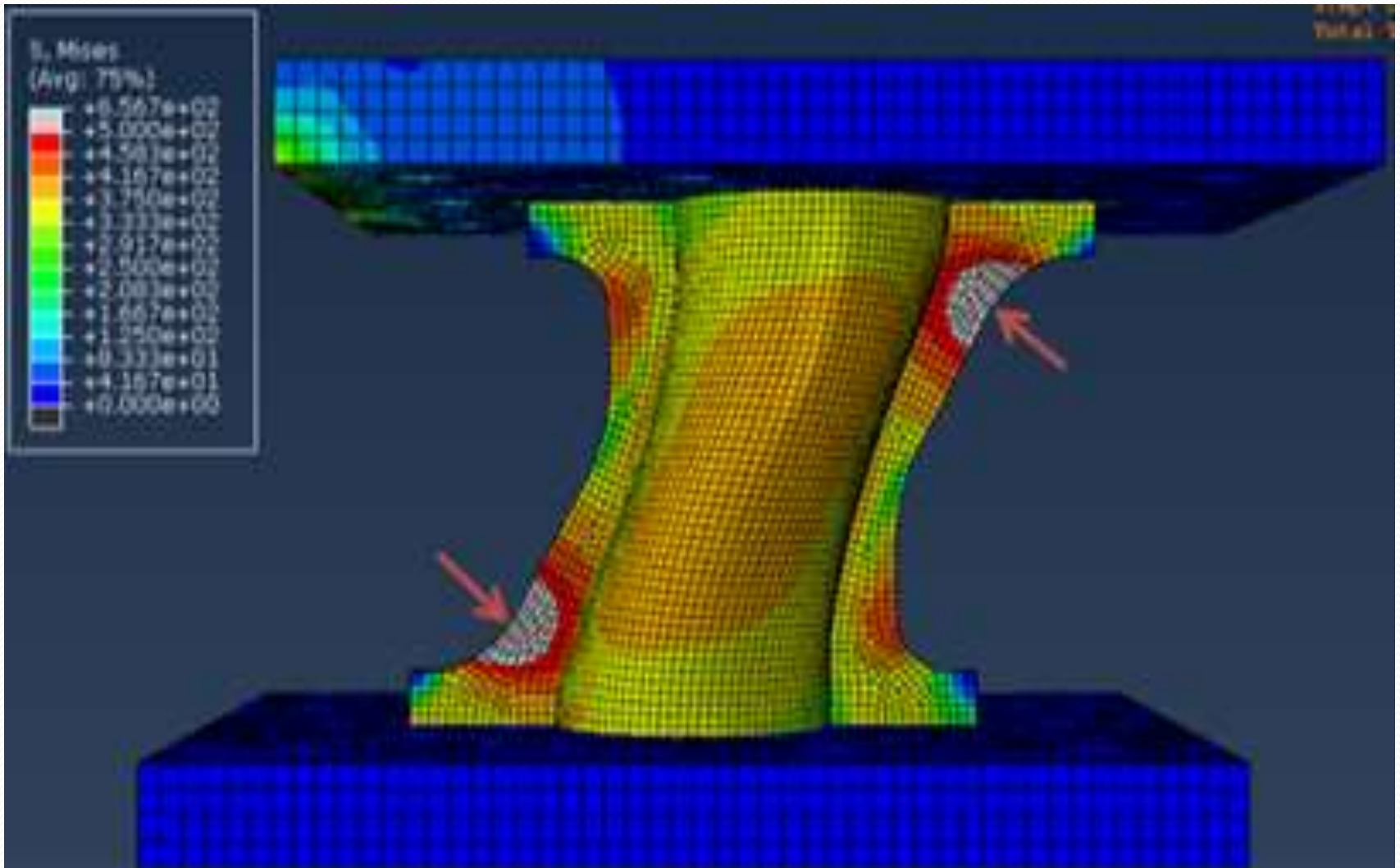


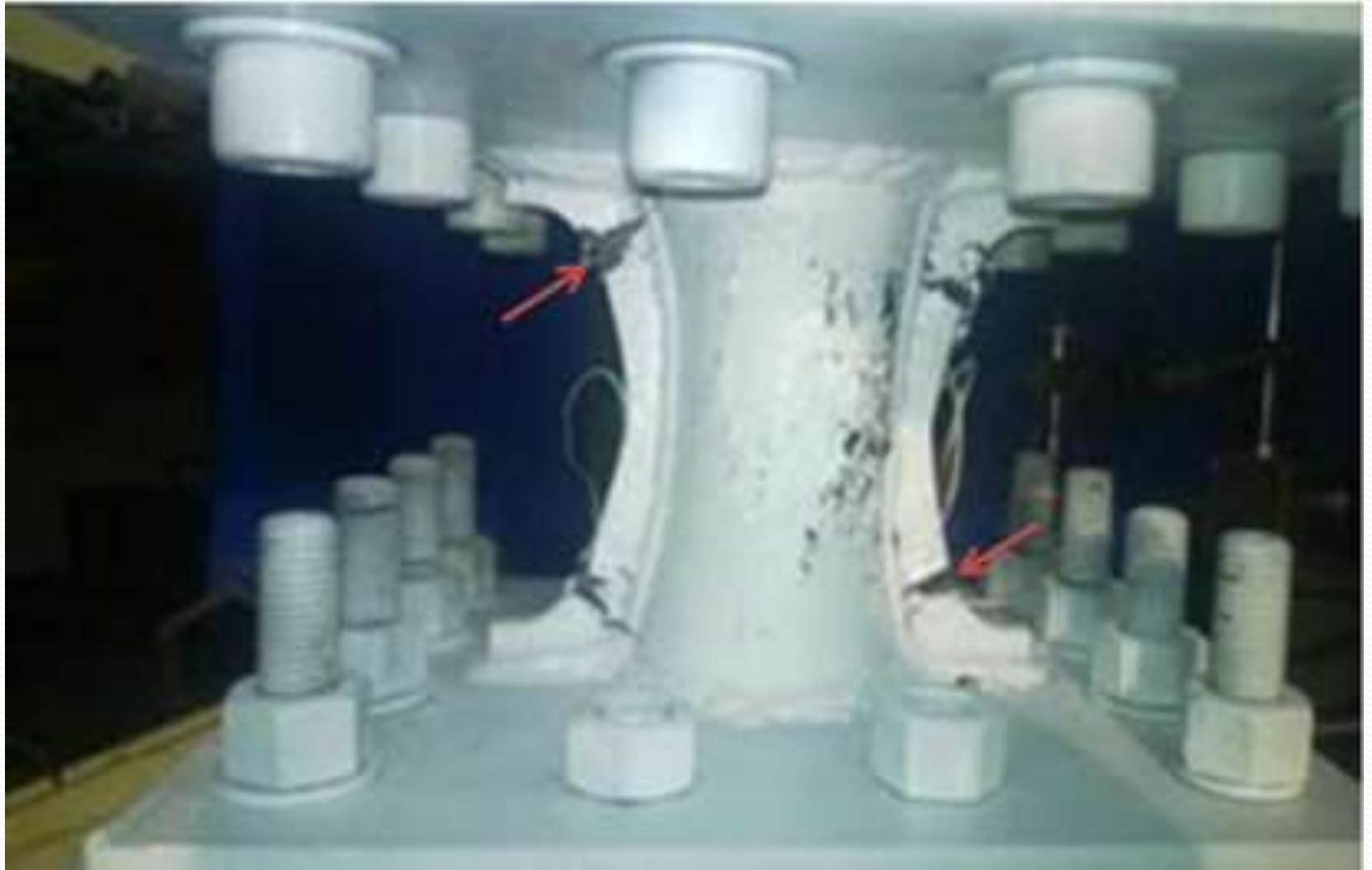
Curved and smoothed to avoid fracture

Welded connections area placed at low stress regions

ODB: Job1.odb Abaqus/Standard 6.11-1 Fri May 29 14:46:37 SE Asia Standard Time 2015

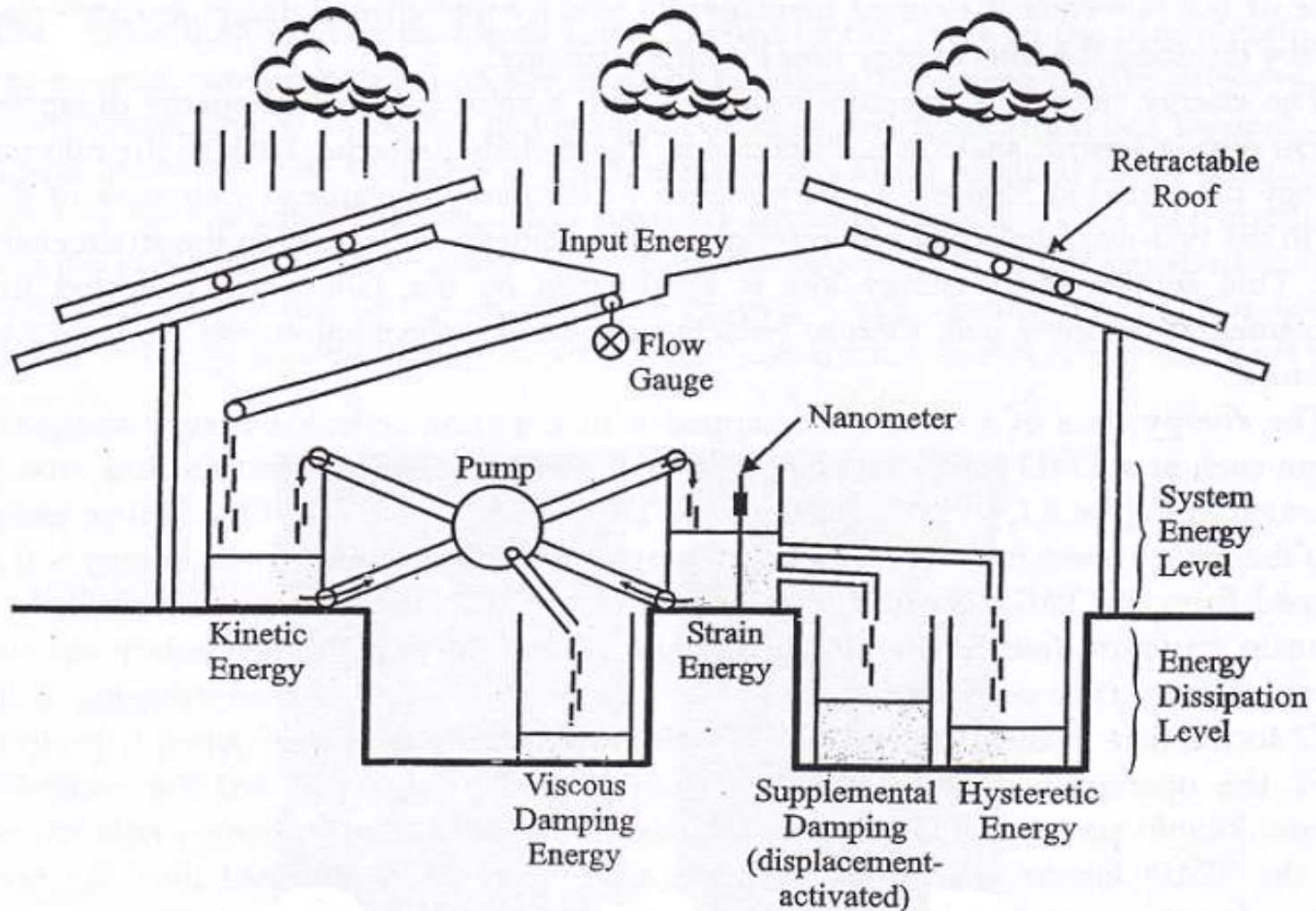
Step: Displacement as load
Increment: 255, Step Time = 25.02
Primary Var: S, Stress
Deformed Var: U, Deformation Scale Factor: +1.000e+00



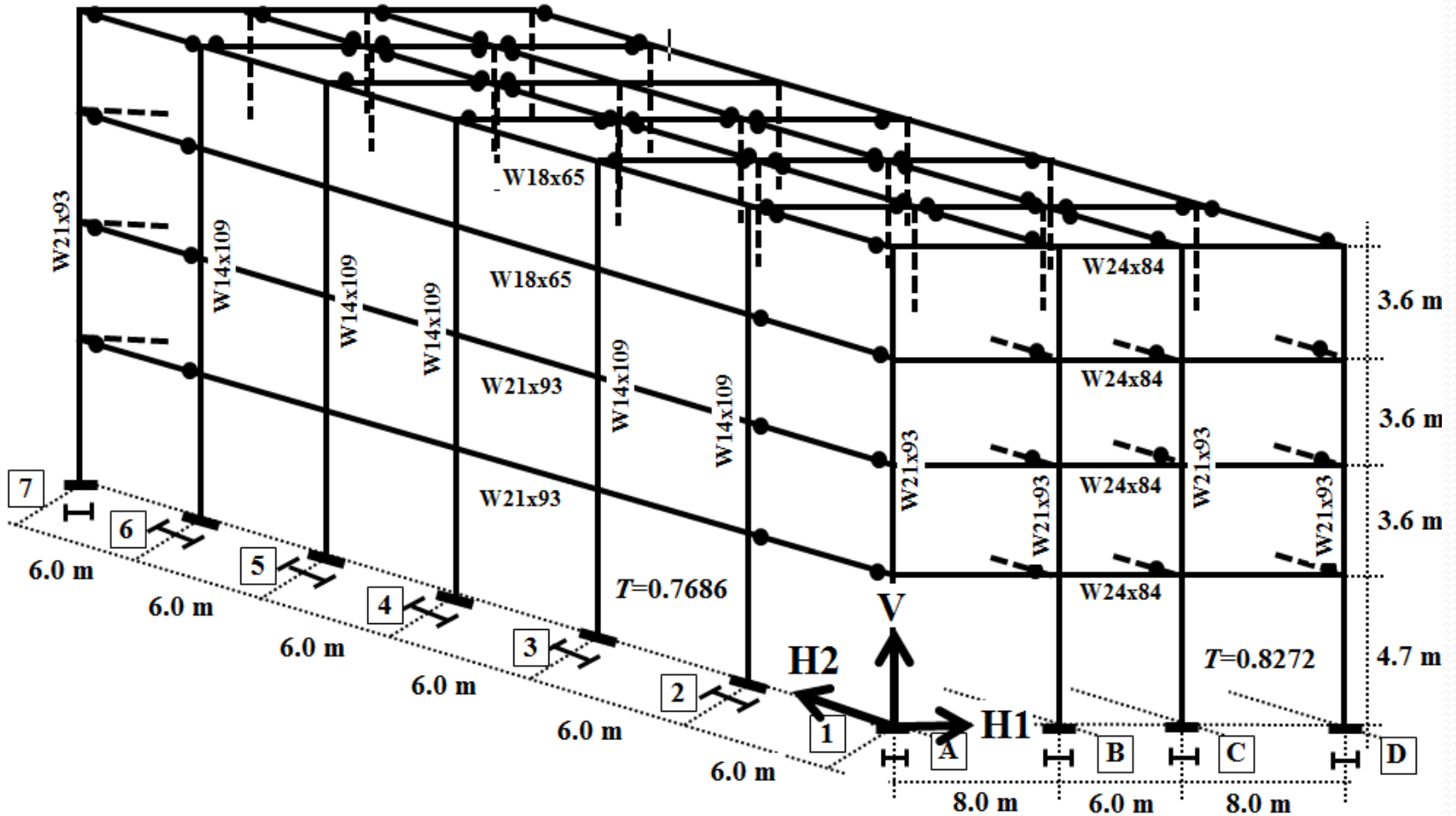


Passive energy dissipation systems

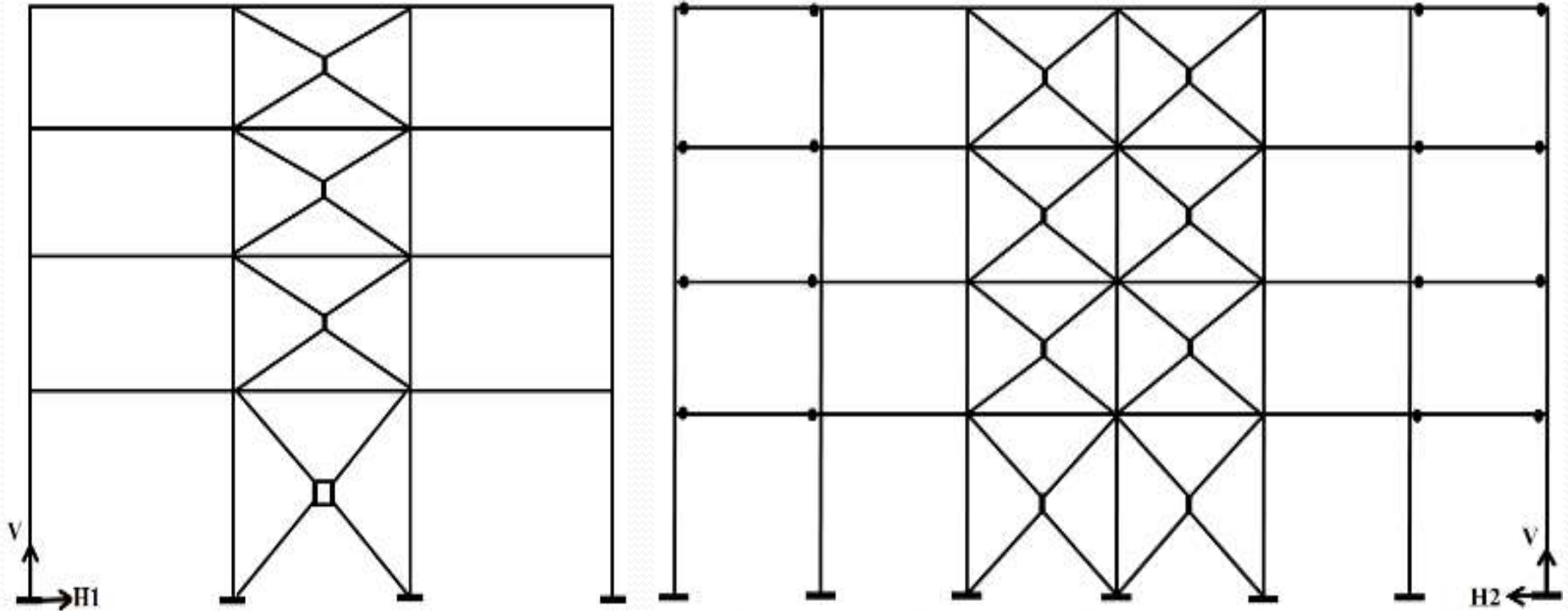
(Christopoulos, C. and Filiatrault, A. (2006))



Damper configuration and earthquake loading

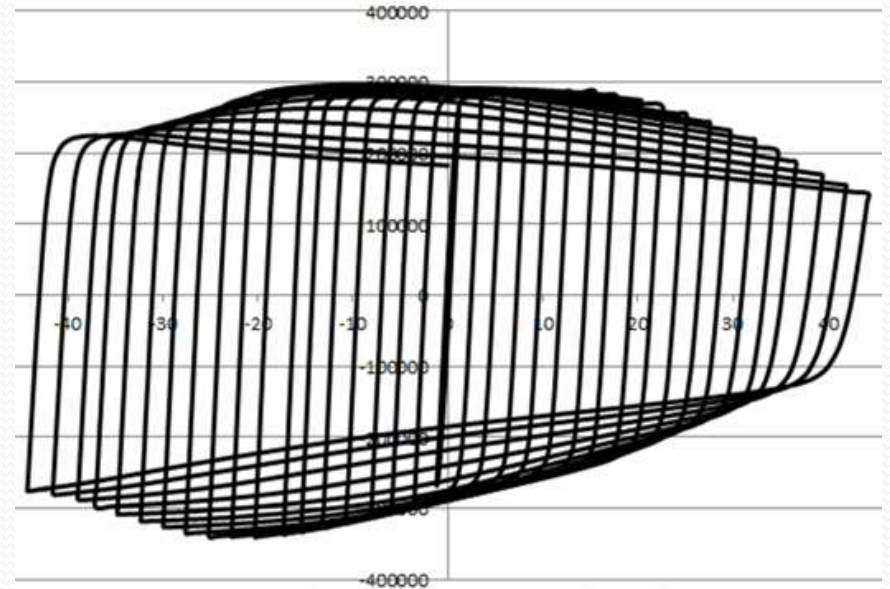
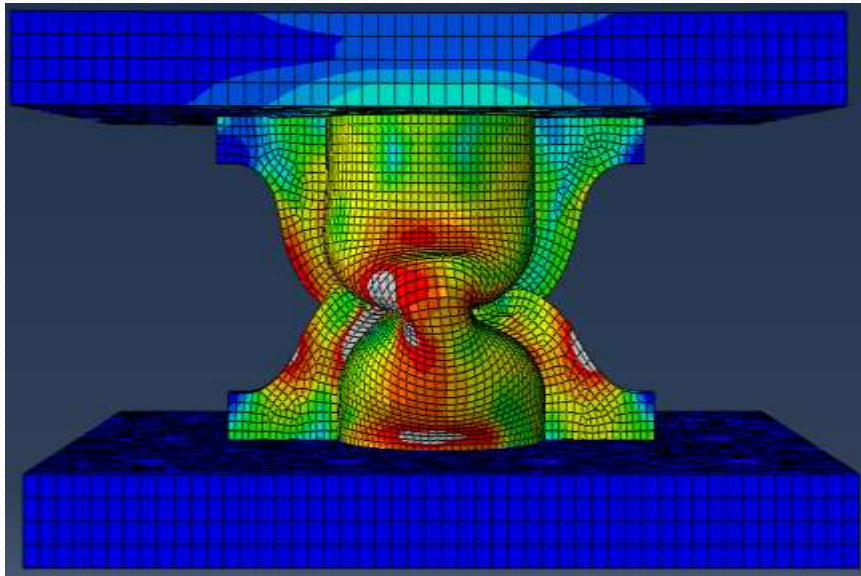


Damper configuration



This configuration is used to minimize the effect of axial forces.

Steel pipe dampers yield when they dissipate input earthquake energy in structures. Yielding steel pipe dampers lose most of their stiffnesses. Steel pipe dampers are weak axially when they yield.



Earthquake time histories

North-South components of the following earthquakes will be used in numerical simulation.

1. El-Centro (California), 1940.
2. Fukushima-Hamadori (Japan), 2011
3. Padang (west Sumatra), 2009
4. Chi-Chi (Taiwan), 1999.

Peak Ground Acceleration of the three other earthquakes were **scaled down** to El-Centro 1940.

Result of the simulation study

COMPONENT PROPERTIES

Materials
Strength Sects
Compound

Inelastic
Elastic
Cross Sects.

Type Seismic Isolator, Rubber Type

Choose type and name to edit an existing component.

Name Vertical_Steel_Pipe_Damper

Length Unit m Force Unit kN

Status Saved.

Basic Relationship
 Bilinear
 Trilinear


Axis 1-2 Symmetry
 Yes No

Deformation Capacities
 Yes No


Strength Capacities
 Yes No

Shear Behavior
Bearing Behavior
Capacities

F = shear and bearing forces. D = shear and bearing deformations.



Axis 3
Axis 2 Axis 1



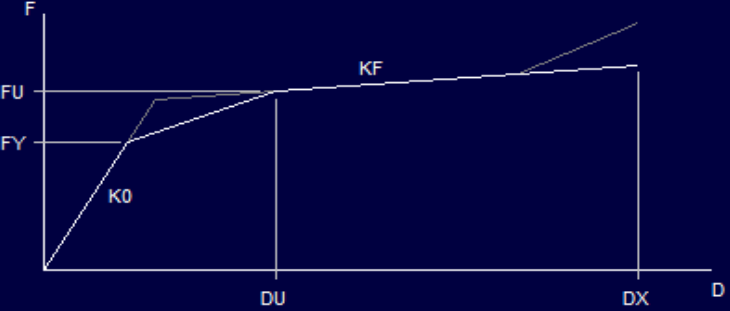
Yield surface
F2 F1

Symmetrical Case

	Along Axis 1	Along Axis 2
K0	110185	
KF	4603.33	
DS		
KS		

Actions	
	Along Axis 2
FY	197.55
FU	463.75

Deformations	
	Along Axis 2
DU	0.007751
DX	0.035



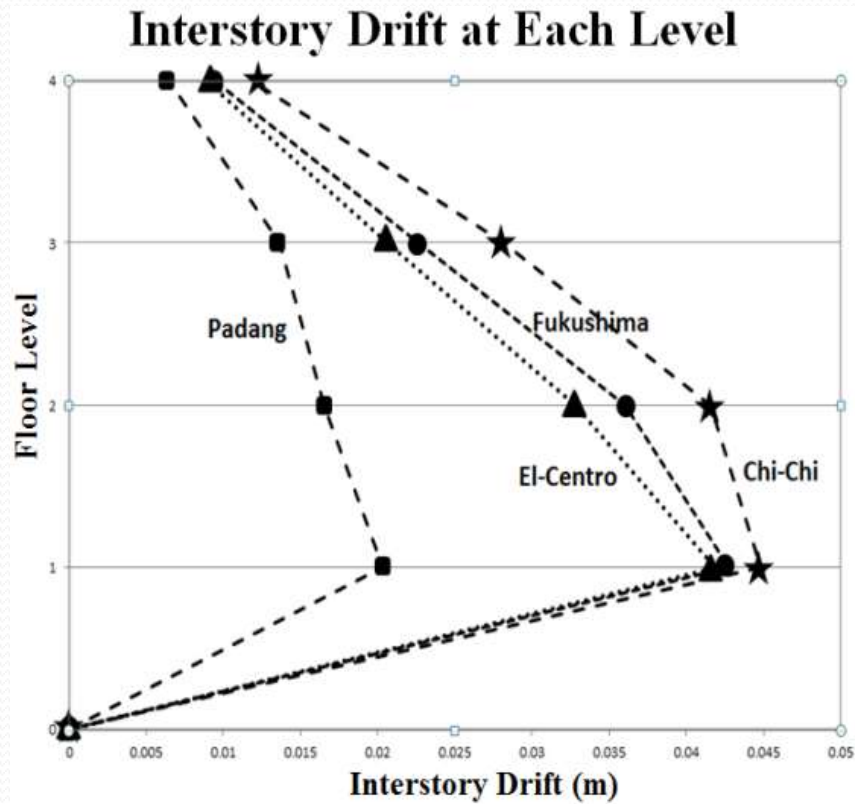
Import Components
Export Components

Selected components of this type.

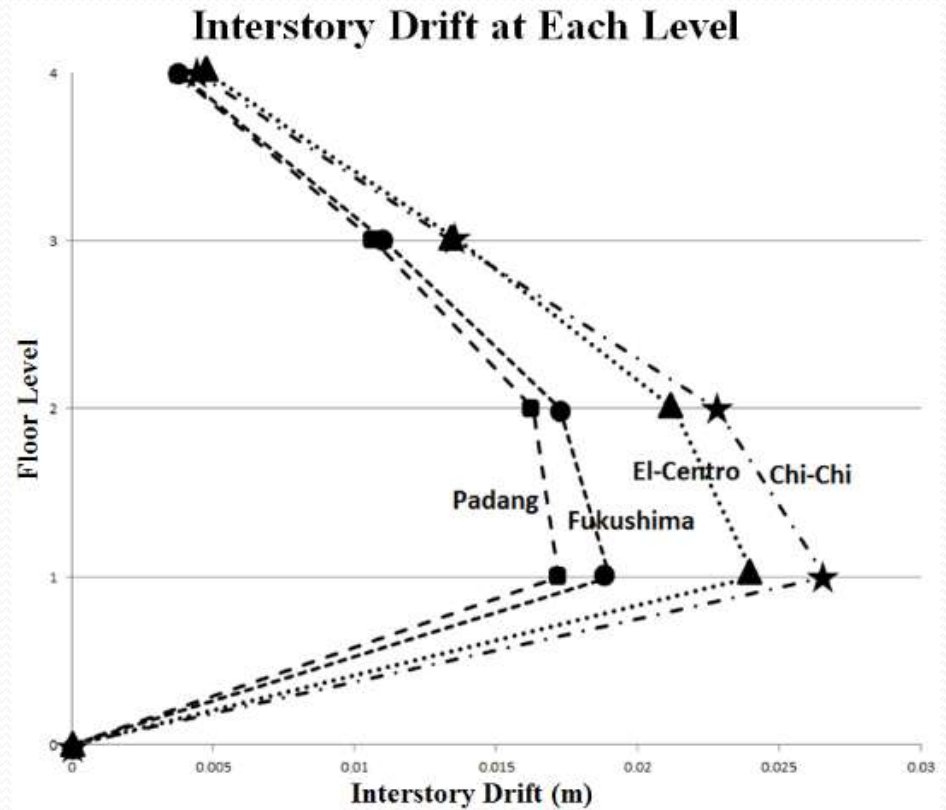
All components of all types.

Four steps in the numerical study of the damper application:

1. Determine the **basic frame structure**.
2. Select the **earthquake time histories** used in the numerical simulation.
3. Determine the **required number of dampers** at each story.
4. Evaluate the **effectiveness of seismic protection** in the frame structure with dampers installed.



(a)



(b)

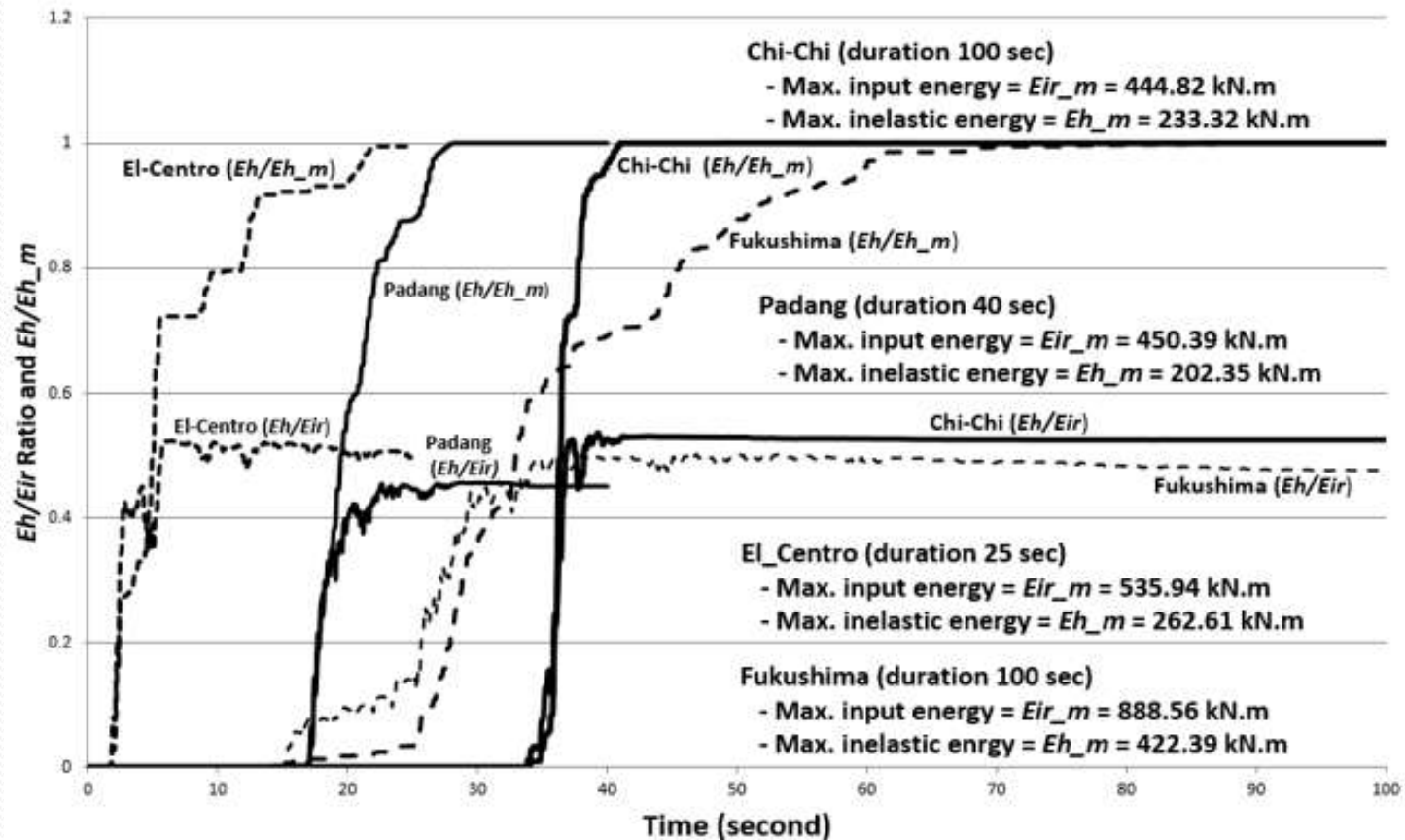
Inter-story drift due to four earthquakes: (a) Frame **without dampers** and (b) Frame **with dampers**

Dissipated inelastic energy in kN.m (along H1)

Group name	Earthquake			
	El-Centro - 1940	Fukushima - 2011	Padang - 2009	Chi-Chi - 1999
Perimeter Columns	0	0	0	0
Perimeter Beams	0	0	0	0
Interior Columns	0	0	0	0
Interior Beams	0	0	0	0
Connection Panel Zones - along H1	0	0	0	0
Connection Panel Zones - along H2	0	0	0	0
Vertical Steel Pipe Dampers	261.49	422.4	202.35	233.29
Bracing HSS-H1-1st floor	0	0	0	0
Bracing HSS-H2-1st floor	0	0	0	0
Bracing HSS-H2-other floors	0	0	0	0
Bracing HSS-H1 other floors	0	0	0	0
Total for All Groups	261.49	422.4	202.35	233.29

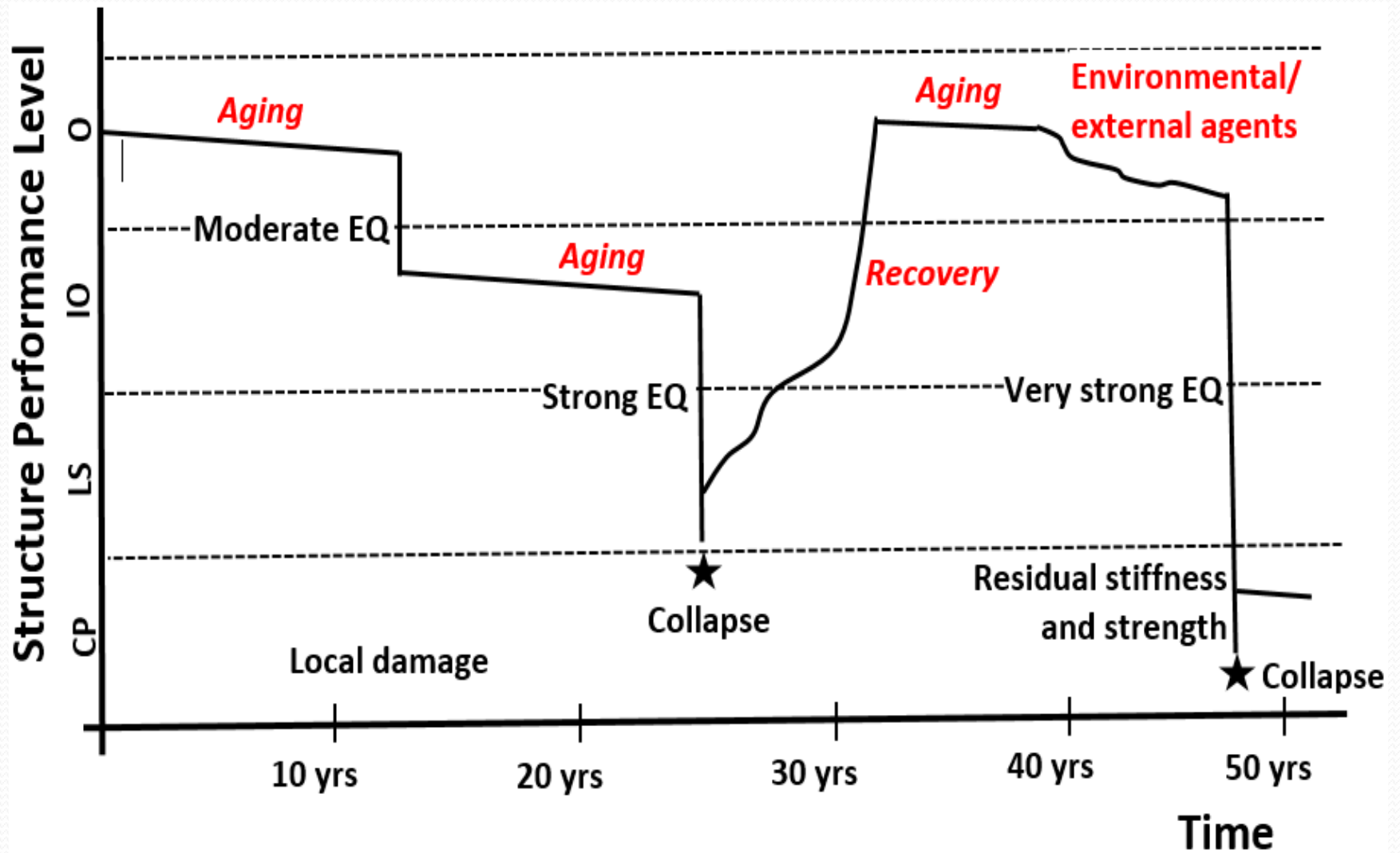
Time history of energy dissipation due to four earthquakes

Inelastic Energy to Input Energy Ratio and Maximum Inelastic Energy

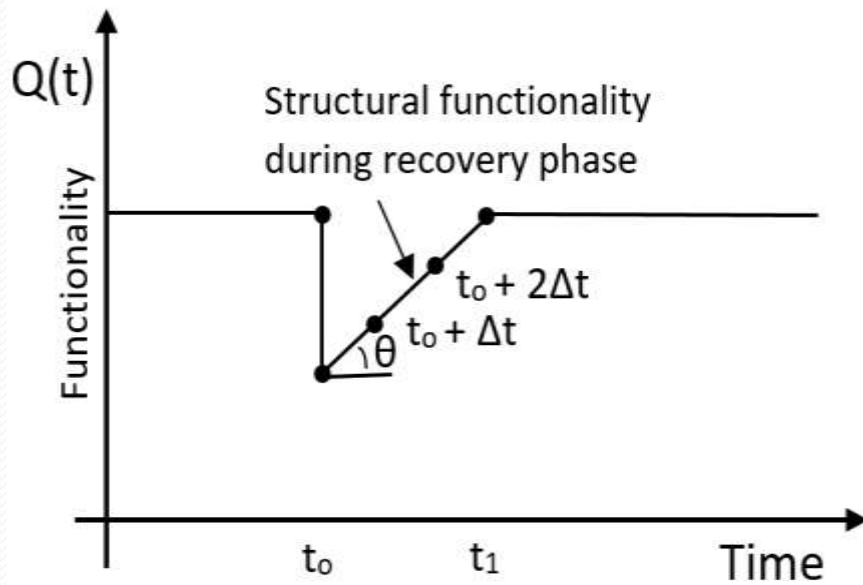


Improving physical resilience of moment resisting frames

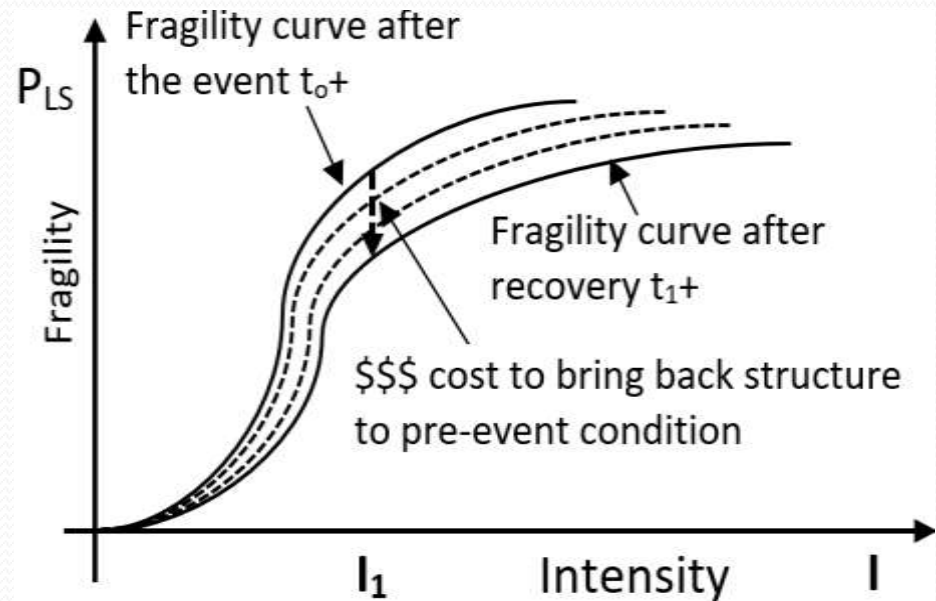
Hazards (corrosion induced damages and earthquakes) pose continuing and significant threats to structures by reducing the capability to withstand the effect of seismic event and to recover efficiently the original functionality of the structures.



Fragility curves



(a)



(b)

Effects of resourcefulness and recovery phase on fragility curve: a) Functionality; b) Fragility curve

Conclusion

The use of steel pipe dampers improves the resiliency (reduces the overall earthquake response) of the moment frames, the installation of steel pipe dampers increases story shears, member forces around the dampers and floor accelerations.